NORTH SANTIAM PAVING CO.

SITE DEVLOPMENT AND ROADWAY CONTRACTORS • CCB# 53247

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Ida Street Tri-Plex Preliminary Drainage Impact Analysis

Stayton, Oregon 97393

Owner:

The Kardboard Box LLC PO Box 617 Stayton, Oregon 97383

Engineer of Record:

Levi Warriner, PE
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PRELIMINARY
OREGON

WARRING

D. WARRING

EXPIRES: 6-30-2026

May 28, 2025

Existing Conditions and Developed Conditions

The site is located on the north side of Ida Street, west of the intersection of Evergreen Avenue and Ida Street. Ida Street is paved with a crown typical section. The existing lot is 10,602 square feet. The lot will have a tri-plex with a paved driveway and associated landscaping.

The City of Portland Stormwater Management Manual allows the simplified method to be used if infiltration rates exceed 2in/hr and there will be less than 10,000 sf of impervious surface. The proposed development will create 6,657 sf of new impervious surface. Previous infiltration testing was completed in 2018 for a potential development on the same site that showed infiltration rates exceeded the 2in/hr. See Appendix A for results. Since both criteria are met for the simplified method, a rain garden was chosen. Per the simplified method, the rain garden size is 10% of the impervious area. The size of the rain garden will be 698 sf. The rain garden will have a maximum ponding depth of 12", before it will overflow towards the driveway approach and onto Ida Street. See attached typical section per City of Portland SMM in Appendix B. See Appendix C for pre and post developed conditions.

APPENDIX A

Location:	Bochsler Addition		Date:	4/18/2018	Test Hole Number:	1 Tri	al Number:	1
Donth to	Bottom of Hole:	ופ	Test Me	othod	Open Pit Falling Head	So	aking Time	
Deptil to	bottom of noie.	3	Test ivie	tiiou	Open Fit Failing nead	7	':07-11:07	

Time	Time Interval (Minutes)	Measurement (Feet)	Drop in Water Level (Feet)	Infiltration Rate (in/hr)	Remarks
11:10	0	2.49	-	-	Filled to 12" water line
11:20	10	2.6	0.11	7.92	
11:30	10	2.70	0.10	7.2	
11:40	10	2.8	0.10	7.2	
11:50	10	2.91	0.11	7.92	
12:00	10	3.01	0.10	7.2	
12:10	10	3.11	0.10	7.2	
			Avg Infiltration Rate (in/hr)	7.44	

Location:	Bochsler Addition		Date:	4/18/2018	Test Hole Number:	1	Trial Number:	2
Depth to	Bottom of Hole:	3'	Test N	/lethod	Open Pit Falling Head			

Time	Time Interval (Minutes)	Measurement (Feet)	Drop in Water Level (Feet)	Infiltration Rate (in/hr)	Remarks
12:20	0	2.53	-	-	Filled with 12"
12:30	10	2.63	0.10	7.2	
12:40	10	2.73	0.10	7.2	
12:50	10	2.83	0.10	7.2	
13:00	10	2.93	0.10	7.2	
13:10	10	3.02	0.09	6.48	
13:20	10	3.11	0.09	6.48	
			Avg Infiltration	6.96	

Avg Infiltration Rate (in/hr)

Location:	ion: Bochsler Addition		Date:	4/18/2018	Test Hole Number:	1	Trial Number:	3
Depth to E	Bottom of Hole:	3'	Test N	/lethod	Open Pit Falling Head			

Time	Time Interval (Minutes)	Measurement (Feet)	Drop in Water Level (Feet)	Infiltration Rate (in/hr)	Remarks
13:27	0	2.55	-	-	Filled with 12"
13:37	10	2.65	0.10	7.20	
13:47	10	2.75	0.10	7.20	
13:57	10	2.84	0.09	6.48	
14:07	10	2.93	0.09	6.48	
14:17	10	3.02	0.09	6.48	
14:27	10	3.11	0.09	6.48	
	_		Avg Infiltration	6.72	

Rate (in/hr)

Location:	: Bochsler Addition		Date:	4/18/2018	Test Hole Number:	2	Trial Number:	1
Donth to	Pottom of Holo	21	Test Method		Open Dit Falling Head		Soaking Time	
Depth to Bottom of Hole:		5	rest ivietnod		Open Pit Falling Head		7:09-11:09	

Time	Time Interval (Minutes)	Measurement (Feet)	Drop in Water Level (Feet)	Infiltration Rate (in/hr)	Remarks
11:12	0	2.66	-	-	Filled to 12" water line
11:22	10	2.76	0.10	7.20	
11:32	10	2.86	0.10	7.20	
11:42	10	2.95	0.09	6.48	
11:52	10	3.04	0.09	6.48	
12:02	10	3.13	0.09	6.48	
12:12	10	3.22	0.09	6.48	
			Avg Infiltration	6.72	

Avg Infiltration Rate (in/hr) 6.72

Location:	Bochsler Addition		Date:	4/18/2018	Test Hole Number:	2	Trial Number:	2	
Depth to	Bottom of Hole:	3'	Test N	/lethod	Open Pit Falling Head				•

Time	Time Interval (Minutes)	Measurement (Feet)	Drop in Water Level (Feet)	Infiltration Rate (in/hr)	Remarks
12:22	0	2.65	-	-	Filled with 12"
12:32	10	2.74	0.09	6.48	
12:42	10	2.83	0.09	6.48	
12:52	10	2.92	0.09	6.48	
13:02	10	3.01	0.09	6.48	
13:12	10	3.1	0.09	6.48	
13:22	10	3.18	0.08	5.76	
	-	•	Avg Infiltration		

Avg Infiltration Rate (in/hr) 6.36

Location:	Bochsler Addition		Date:	4/18/2018	Test Hole Number:	2	Trial Number:	3	
Depth to B	Sottom of Hole:	3'	Test N	Nethod	Open Pit Falling Head				

Time	Time Interval (Minutes)	Measurement (Feet)	Drop in Water Level (Feet)	Infiltration Rate (in/hr)	Remarks
13:29	0	2.68	-	-	Filled with 12"
13:39	10	2.77	0.09	6.48	
13:49	10	2.86	0.09	6.48	
13:59	10	2.95	0.09	6.48	
14:09	10	3.03	0.08	5.76	
14:19	10	3.11	0.08	5.76	
14:29	10	3.19	0.08	5.76	
			Avg Infiltration	6.12	

Avg Infiltration
Rate (in/hr)

Location:	Bochsler	Addition	Date:	4/18/2018	Test Hole Number:	3	Trial Number:	1
Depth to Bottom of Hole:		ופ	Test Method		Open Pit Falling Head		Soaking Time	
		3			Open Fit Falling Head		7:10-11:10	

Time	Time Interval (Minutes)	Measurement (Feet)	Drop in Water Level (Feet)	Infiltration Rate (in/hr)	Remarks
11:14	0	2.61	-	-	Filled to 12" water line
11:24	10	2.74	0.13	9.36	
11:34	10	2.87	0.13	9.36	
11:44	10	3	0.13	9.36	
11:54	10	3.12	0.12	8.64	
12:04	10	3.24	0.12	8.64	
12:14	10	3.36	0.12	8.64	
			Avg Infiltration	9.00	

Rate (in/hr)

Location:	Bochsler	Addition	Date:	4/18/2018	Test Hole Number:	3	Trial Number:	2
Depth to Bottom of Hole: 3'		Test N	/lethod	Open Pit Falling Head				

Time	Time Interval (Minutes)	Measurement (Feet)	Drop in Water Level (Feet)	Infiltration Rate (in/hr)	Remarks
12:25	0	2.6	-	-	Filled with 12"
12:35	10	2.73	0.13	9.36	
12:45	10	2.85	0.12	8.64	
12:55	10	2.97	0.12	8.64	
13:05	10	3.08	0.11	7.92	
13:15	10	3.19	0.11	7.92	
13:25	10	3.3	0.11	7.92	
	-	-	Avg Infiltration		

Avg Infiltration Rate (in/hr) 8.40

Location:	Bochsle	r Addition	Date:	4/18/2018	Test Hole Number:	3	Trial Number:	3
Depth to B	Sottom of Hole:	3'	Test N	/lethod	Open Pit Falling Head			

Time	Time Interval (Minutes)	Measurement (Feet)	Drop in Water Level (Feet)	Infiltration Rate (in/hr)	Remarks
13:34	0	2.62	-	-	Filled with 12"
13:44	10	2.74	0.12	8.64	
13:54	10	2.85	0.11	7.92	
14:04	10	2.96	0.11	7.92	
14:14	10	3.07	0.11	7.92	
14:24	10	3.18	0.11	7.92	
14:34	10	3.29	0.11	7.92	
			Avg Infiltration	8.04	

Rate (in/hr)

APPENDIX B

3.2.9 Rain Gardens



Rain gardens are landscape depressions designed to capture, store, and infiltrate stormwater runoff. They do not include underdrains, liners, or formal walls, and the native soils are amended rather than replaced with an imported soil blend.

Design

Site Suitability: The measured infiltration rate of the soil must be at least 2 inches per hour.

Catchment Area: Stormwater runoff from a catchment area of up to 10,000 ft² can be managed by multiple rain gardens. Up to 5,000 ft² of impervious area can drain to an individual rain garden.

Setbacks: Table 3-10 shows standard setback requirements for rain gardens. Setbacks are measured from the typical high water level in the rain garden.

Table 3-10. Standard Setbacks for Rain Gardens¹

Setback to	Distance (ft)
Property lines along parcels ²	5
Foundations	10
Drainfields downslope of the rain garden	100
Top of grade breaks > 3 ft high and top of slopes ≥ 25%, downslope of the rain garden	5 times the slope height up to 100 ft
Top of slopes > 50% and > 10 ft high, downslope of the rain garden	200

^{1.} These setbacks apply to both onsite and offsite foundations, drainfields, grade breaks, and slopes.

Inflow: Conveyance of stormwater runoff to the rain garden through flexible downspout extensions is prohibited. Conveyance options include the following:

- An aboveground, gutter-grade downspout extension that drains directly to areas that can accommodate stormwater runoff without crossing walkways or draining onto driveways, patios, or other impervious surfaces.
- An underground downspout extension, daylighting into the rain garden, that is watertight within the 2-ft or 6-ft setback to the building.
- A rock-filled drainage channel that is lined with waterproof sheeting (e.g., 30-mm ethylene propylene diene terpolymer (EPDM)) within the 2-ft or 6-ft setback to the building.

Access: The design must provide safe access for maintenance of the rain garden and access to adjacent buildings or infrastructure. Paths, gates, and covers must be safely accessible.

Pollution Prevention: Do not use chemically treated products with the potential for leaching pollutants (e.g., railroad ties, treated lumber, recycled crushed asphalt, and galvanized metals). Minimize the need for toxic or potentially polluting materials

^{2.} No setbacks are required for property lines with the right-of-way.

such as herbicides, pesticides, and fertilizers. These materials create the risk of spills, misuse, and draining or leaching of pollutants into the rain garden and surrounding areas.

Sizing: The footprint of the rain garden, measured at the elevation of the overflow and including side slopes, must be at least 10% of the impervious area draining to it.

Dimensions and Slopes:

- Minimum freeboard (vertical distance between the overflow rim and the top of rain garden rim): 2 inches.
- Ponding depth (vertical distance between the top of the soil and the elevation of an overflow rim, such as a standpipe grate): 12 inches.
- Maximum side slope: 3 horizontal to 1 vertical (3:1). A side slope of 2:1 may be allowed if there are erosion control protections such as vegetative cover and larger boulders.
- Minimum bottom width: 2 feet.
- Grades must slope away from foundations and neighboring properties.

Overflow: The design should indicate where excess stormwater runoff will safely flow in heavy rainfall. Overflow routes should be planted or covered in rock to prevent erosion. A piped overflow is allowed if an overflow route is not feasible.

Erosion Control at Stormwater Entrances: Install a layer of river rock or flagstones at stormwater entrances to dissipate the energy of incoming stormwater runoff in cases where inflow is concentrated. Plastic splash blocks are not allowed.

Soil Amendment: Amend the native soil with 3 inches of yard-debris compost and blend to a depth of 12 inches below grade.

Mulch (optional): The surface can be mulched with 2-3 inches of dark (aged) medium hemlock mulch. Manure-based compost is prohibited. Mulch should be weed-free and applied 2 to 3 inches thick to fully cover the soil between plants.

Planting: See Section 3.2.12.1.

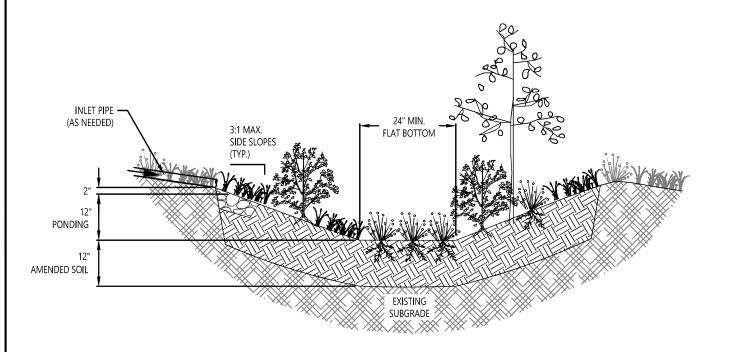
Construction Guidance

See Section 3.2.12.2.

Typical Detail

See Facility Details - Simplified Approach for the typical detail.

• SW-112 Rain garden



- 1. Setbacks: 10 feet from edge of facility to building foundations; 5 feet to property lines; zero setback from public right-of-way where approved.
- 2. Overflow: A rain garden must include an overflow route that safely directs runoff to a disposal point in heavy rainfall. Overflow routes must drain away from building foundations and adjacent properties. Overflow routes must be planted or covered with rock to limit erosion.
- 3. Amended Soil: Amend native soil with 3 inches of yard debris compost, blend to a depth of 12 inches.
- 4. Plants: Minimum container size is 1 gallon. In inundated areas (Zone A), install 80 herbaceous plants or 72 herbaceous plants and 4 small shrubs per 100 sf. In drier upland areas above the rim of the overflow (Zone B), install 7 large or small shrubs and 70 groundcover plants. If project area is over 200 sf consider installing a tree.
- 5. Entrance Erosion Control: Install riprap at stormwater entrances to reduce erosive power of incoming water.
- 6. Mulch (Optional): The surface can be mulched with 2 inches of dark (aged) medium hemlock mulch.
- 7. See SWMM Chapter 3 for guidance.

INSPECTION INFORMATION

SCHEDULE INSPECTION (IVR): 503-823-7000, CODE #487. 2 INSPECTIONS REQUIRED.

- Inspection #1: Location, size, depth
- Inspection #2: Soil, plants, elevations, inlet protection

- DRAWING NOT TO SCALE



STORMWATER MANAGEMENT TYPICAL DETAILS FOR PRIVATE PROPERTY

RAIN GARDEN

SW-112

APPENDIX C

